



## Including Life Cycle Assessment for decision-making in controlling wastewater nutrient removal systems



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### ABSTRACT

This paper focuses on the use of Life Cycle Assessment (LCA) to evaluate the performance of seventeen control strategies in wastewater treatment plants (WWTPs). It tackles the importance of using site-specific factors for nutrient enrichment when decision-makers have to select best operating strategies. Therefore, the LCA evaluation is repeated for three different scenarios depending on the limitation of nitrogen (N), phosphorus (P), or both, when evaluating the nutrient enrichment impact in water bodies. The LCA results indicate that for treated effluent discharged into N-deficient aquatic systems (e.g. open coastal areas) the most eco-friendly strategies differ from the ones dealing with discharging into P-deficient (e.g. lakes and rivers) and N&P-deficient systems (e.g. coastal zones). More particularly, the results suggest that strategies that promote increased nutrient removal and/or energy savings present an environmental benefit for N&P and P-deficient systems. This is not the case when addressing N-deficient systems for which the use of chemicals (even for improving N removal efficiencies) is not always beneficial for the environment. A sensitivity analysis on using weighting of the impact categories is conducted to assess how value choices (policy decisions) may affect the management of WWTPs. For the scenarios with only N-limitation, the LCA-based ranking of the control strategies is sensitive to the choice of weighting factors, whereas this is not the case for N&P or P-deficient aquatic systems.

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## 1. Introduction

Decisions about wastewater nutrient removal systems have traditionally been driven by considerations of technical aspects and cost-benefit analyses. In order to assess sustainability it is essential also to incorporate environmental and social aspects. In this line, several methods for sustainability assessment of wastewater treatment technologies have been proposed and evaluated in literature (e.g. Muga and Mihelcic, 2008; Guinée et al., 2011). Focussing on environmental performance, Life Cycle Assessment (LCA) (ISO 14040, 2006) is an accepted tool that has also been used to evaluate potential environmental impacts from environmental

processes (Finnveden et al., 2009) including wastewater treatment processes throughout their whole life cycle (Guest et al., 2009; Kalbar et al., 2012). The success of this method is demonstrated by the large number of published studies which have applied LCA to the wastewater treatment field (Corominas et al., 2013). The published studies so far, have been applied to estimate the impact of different wastewater treatment plants (WWTPs) and to compare conventional and new wastewater treatment technologies.

Complex wastewater nutrient removal systems require sophisticated monitoring and control systems that optimize overall process performance and account for daily and seasonal process variability. The optimization of treatment technologies dealing with nitrogen and phosphorus removal has never been evaluated in terms of environmental performance using LCA, probably because of the technical difficulty of evaluating multiple strategies at full scale. An alternative to intensive full-scale optimization is the use of models. These, are cost-effective tools for the evaluation of control strategies as has been demonstrated by the IWA Task group of benchmarking of control strategies (Gernaey et al., 2013).

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Hence, the combination of mechanistic models for evaluating control strategies with LCA tools can bridge the gap between process control and environmental performance (Flores-Alsina et al., 2010).

Linking LCA results with decision-making is a challenging task. First, LCA has traditionally been a site and time-independent/generic tool and for impact categories such as global warming (GW) a site-generic approach is justifiable. However, other impact categories are site-dependent (e.g. nutrient enrichment, NE) and the generic models most probably provide a far from realistic description of the impacts. Therefore, LCA research is moving towards developing methodologies that include the importance of local conditions and set country-specific or site-dependent characterization factors (e.g. Gallego et al., 2010; Basset-Mens et al., 2006; Azevedo et al., 2012). Second, LCA methodology proposes a weighting step, which allows for consideration of the relative importance of the different impact categories. Most studies present the results without weighting leaving the final interpretation to the decision-maker. We believe that incorporating an evaluation of the importance of the weighting factors in the discussion of the LCA results would facilitate the task of the decision-makers.

The goal of this paper is to present a methodology to evaluate the environmental impacts of enhanced process performance strategies applied to wastewater nutrient removal systems. The assessment of site-specific conditions for the NE impact category and the assessment of the importance of weighting factors are key issues addressed in this paper.

## 2. Process and nutrient removal strategies description

### 2.1. Process

The Neptune Simulation Benchmark (NSB) is the virtual WWTP under study (Fig. 1) which serves 80,000 population equivalents and comprises the water treatment and the sludge treatment lines. The design of the water treatment line was conducted following the Metcalf & Eddy guidelines (Tchobanoglous et al., 2003). First, there is a primary settler of 900 m<sup>3</sup>, which reaches on average 50% TSS removal. Then, biological treatment was designed for an average flow rate of 22,938 m<sup>3</sup> d<sup>-1</sup> and organic, nitrogen and phosphorous loads (just after primary settling) of 12,200 kg COD d<sup>-1</sup>, 1140 kg N d<sup>-1</sup> and 215 kg P d<sup>-1</sup> respectively. The biological treatment configuration is the A<sub>2</sub>O (Tchobanoglous et al., 2003) comprised of seven reactors in series (tank An1 & An2 are anaerobic with a volume of 1000 m<sup>3</sup> each, tanks Ax1 and Ax2 are anoxic with a volume of 1500 m<sup>3</sup> each and tanks Ox1, Ox2 and Ox3 are aerobic with a volume of 3000 m<sup>3</sup> each). Ax1 and Ox3 are linked by means of an internal recycle of 108,000 m<sup>3</sup> d<sup>-1</sup>. The system allows for the addition of chemicals to improve nitrogen and phosphorus removal. Sodium acetate can be added in An1 when carbon source is a limiting factor for denitrification and phosphorus removal. Ferric chloride can be added in Ox3 to promote the precipitation of phosphorus. The secondary settler has a surface area of 1500 m<sup>2</sup> and a total volume of 6000 m<sup>3</sup>. Part of the settled sludge is recycled to An1 (22,600 m<sup>3</sup> d<sup>-1</sup>) and part is wasted (400 m<sup>3</sup> d<sup>-1</sup>) for further

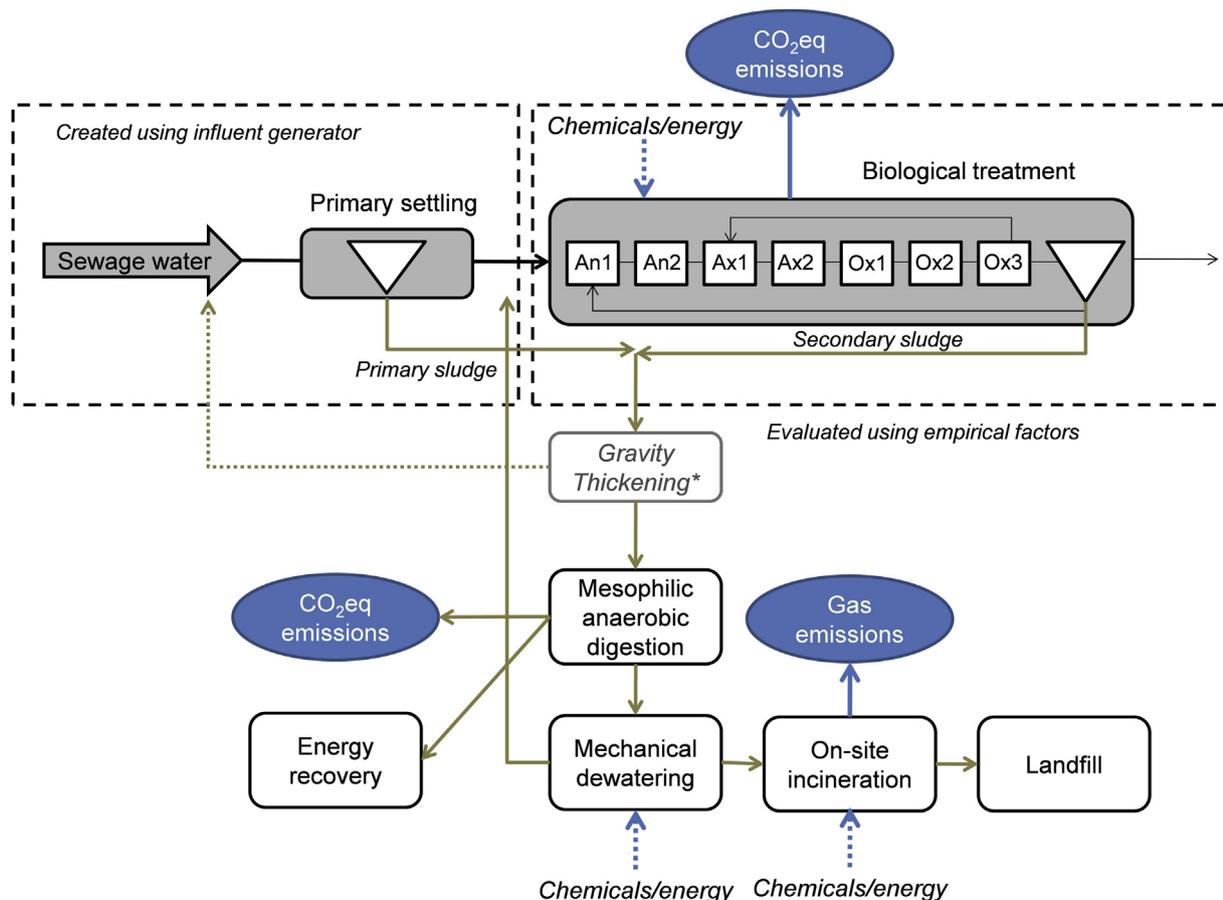


Fig. 1. Neptune simulation benchmark process schematic. Grey boxes correspond to a deterministic dynamic model and white boxes to simplified model using empirical factors. \*Gravity thickening is assumed not to have a significant impact on the LCA results and is therefore not included in the evaluation.

sludge treatment line together with the primary settler waste. Sludge treatment starts with gravity thickening, followed by mesophilic anaerobic digestion. The biogas produced is burnt in a co-generator to recover energy. Afterwards, the sludge goes to mechanical dewatering and the supernatant obtained is recycled just before An1. After dewatering, the sludge is burnt in an on-site incinerator that uses electricity produced in the co-generator. Finally, the ashes are transported to landfill. Several chemicals are applied during sludge treatment for promoting flocculation, precipitation and neutralization for pH adjustment. The design of the sludge treatment line has been taken from the Neptune project (EU FP6) as documented in Larsen et al. (2010).

## 2.2. Process models

### 2.2.1. Wastewater treatment line

Modelling and simulation works were performed using the WEST<sup>®</sup> modelling/simulation environment (mikebyDHI.com). The EAWAG Activated Sludge Model (ASM) No 3 Bio-P was chosen as bio-chemical model (Rieger et al., 2001). This model has 19 state variables and describes (bio) chemical phosphorus removal, nitrification and denitrification in activated sludge systems by means of a set of nonlinear differential equations. The model was extended to include chemical precipitation of phosphorus as in ASM2d (Henze et al., 2000). The double exponential velocity function defined in Takács et al. (1991), based on the solids flux concept was selected as a fair representation of the settling process, using a 10 layer discretization. Kinetic parameters were adjusted according to the influent temperature using the Arrhenius equation. The default parameters for the activated sludge and the settling model can be found in Rieger et al. (2001) and Germaey et al. (2013) respectively, while the parameters for phosphorus precipitation kinetics were taken from Germaey et al. (2002). It is important to highlight that the settling characteristics were assumed to be constant along the case study although the authors are aware that the floc characteristics may change in systems with chemical precipitation. Besides the use of the bio-chemical mechanistic model, direct greenhouse gases emissions (GHG) were estimated using simple comprehensive models (Table 1). CO<sub>2</sub> emissions generated during the oxidation of non-biogenic organics were modelled as described in

Corominas et al. (2012). Nitrous oxide (N<sub>2</sub>O) emissions generated during treatment were also included using the factor 0.09% of TN removed emitted as N<sub>2</sub>O–N proposed in Ahn et al. (2010) for BNR plants. Also, energy models were considered in the biological treatment including aeration energy demand (for organic matter and nitrogen removal), mixing energy (of the activated sludge tanks) and pumping energy (for the internal and external recirculations) using the equations proposed in Germaey et al. (2013) (Table 1). The load of sludge produced (SP) in the biological treatment that goes to the sludge line was calculated based on the amount of solids accumulated in the plant and from the solids wasted from the primary and secondary clarifiers.

### 2.2.2. Sludge treatment line

The sludge produced in the wastewater treatment line (in mass per day) is the input of the sludge treatment model which consists of empirical factors to estimate i) biogas production, non-methane volatile organic compounds (NMVOCs), energy consumption and energy recovery for the anaerobic digester, ii) chemicals consumption for dewatering, iii) emissions and chemicals consumption for on-site incineration and iv) emissions of organic and inorganic compounds at landfill. The sludge incineration factors were taken from the ARA Hard Sludge Incineration Plant in Winterthur, Switzerland (Larsen et al., 2010). A summary of the empirical factors is presented in Table 2.

## 2.3. Simulations

Plant performance evaluation was based on 609 days simulated influent data generated using the influent generator developed in Germaey et al. (2011). All dynamic simulations (609 days) were preceded by steady state simulations (200 days). Only the data generated during the last 364 days of the dynamic simulations were used for plant performance evaluation and the output sampling was set to 15 min (Germaey et al., 2013).

## 2.4. Enhanced process performance strategies

Seventeen nutrient removal strategies were evaluated in this study. A reference strategy (A<sub>1</sub>) was defined where the system

**Table 1**  
Details of the NSB model. Water line.

Process	Factor/equation/model	Source
Primary settling	Otterpohl and Freund	Otterpohl and Freund (1992)
Biological secondary treatment	ASM3 Bio-P	Rieger et al. (2001)
Sludge production (kg d <sup>-1</sup> )	$SP = \frac{1}{t_{obs} \cdot 1000} \cdot (M_{TSS}(t_{end}) - M_{TSS}(t_{ini})) + \int_{t_{ini}}^{t_{end}} TSS_{prim}(t) \cdot Q_{prim}(t) \cdot dt + \int_{t_{ini}}^{t_{end}} TSS_{sec}(t) \cdot Q_{sec}(t) \cdot dt$	Germaey et al. (2013)
Secondary settling	Takács	Takács et al. (1991)
Aeration energy for secondary treatment (kWh d <sup>-1</sup> )	$AE = \frac{S_{O_2}^{15}}{t_{obs} \cdot 1.8 \cdot 1000} \int_{t_{ini}}^{t_{end}} \sum_{i=1}^{i=7} (V_i \cdot K_L a_i^{15}(t)) \cdot dt$	Germaey et al. (2013)
Mixing energy for secondary treatment (kWh d <sup>-1</sup> )	$ME = \frac{24}{t_{obs}} \int_{t_{ini}}^{t_{end}} \sum_{i=1}^{i=7} \left[ \begin{array}{l} \text{for } t \text{ when } K_L a_i(t) < 20 \text{ d}^{-1} \text{ then } ME_{unit} \cdot V_i \\ \text{for } t \text{ when } K_L a_i(t) \geq 20 \text{ d}^{-1} \text{ then } 0 \end{array} \right] \cdot dt$	Germaey et al. (2013)
Pumping energy for secondary treatment (kWh d <sup>-1</sup> )	$PE = \frac{1}{t_{obs}} \int_{t_{ini}}^{t_{end}} (PE_{Q_{int}} \cdot Q_{int} + PE_{Q_r} \cdot Q_r + PE_{Q_w} \cdot Q_w) dt$	Germaey et al. (2013)
N <sub>2</sub> O, Biological treatment	0.0009 kg N <sub>2</sub> O kg TN removed <sup>-1</sup>	Ahn et al. (2010)
CO <sub>2</sub> , oxidation of sodium acetate (kg CO <sub>2</sub> d <sup>-1</sup> )	$CO_2, BOD_{ox} = [BOD_{ox} - 1.42 \cdot \left( \frac{Y}{1+k_d \cdot SRT} \right) \cdot BOD_{ox}] \cdot f_{CO_2/O_2}$	Corominas et al. (2012)

For AE,  $t_{obs}$  is the period of simulation (d) ( $t_{end} - t_{ini}$ ),  $V_i$  is the volume of reactor  $i$  (m<sup>3</sup>),  $K_L a_i^{15}$  is the mass transfer coefficient in reactor  $i$  recalculated to 15 °C (d<sup>-1</sup>), and  $S_{O_2}^{15}$  is the saturation oxygen concentration at 15 °C (8 g (-COD) m<sup>-3</sup>). For SP,  $M_{TSS}$  is the total suspended solids in the reactors,  $TSS_{prim}$  and  $TSS_{sec}$  are the total solids concentrations in the primary and secondary settler underflows,  $Q_{prim}$  and  $Q_{sec}$  are the wastage flow rates in the primary and secondary settlers. For ME,  $ME_{unit}$  is the mixing energy consumption for the activated sludge tanks, which is set to 0.005 kW m<sup>-3</sup>. PE is calculated as a weighted averaged sum of the internal recycle flow rate  $Q_{int}$ , the external recycle flow rate  $Q_r$ , the waste sludge flow rate  $Q_w$ .  $PE_{Q_{int}}$ : 0.004 kWh m<sup>-3</sup>,  $PE_{Q_r}$ : 0.008 kWh m<sup>-3</sup>,  $PE_{Q_w}$ : 0.050 kWh m<sup>-3</sup>.  $BOD_{ox}$  is the BOD load (kg d<sup>-1</sup>) corresponding to sodium acetate addition,  $Y$  is the yield (0.84 kg VSS kg BOD removed<sup>-1</sup>),  $SRT$  is the sludge retention time (d),  $k_d$  is the endogenous decay coefficient (0.05 d<sup>-1</sup>), and  $f_{CO_2/O_2}$  corresponds 1.1 kg of CO<sub>2</sub> produced per 1 kg O<sub>2</sub> consumed in the BOD oxidation.

**Table 2**  
Details of the NSB model. Sludge line.

Type	Process	Factor/equation/model	Units	
Electricity consumption	Anaerobic digestion	70	kWh tDM <sup>-1</sup>	
	On-site incineration	330 (110 from digester + 220 from external supply)	kWh tDM <sup>-1</sup>	
Electricity production	Dewatering	20	kWh tDM <sup>-1</sup>	
	Anaerobic digestion	500	kWh tDM <sup>-1</sup>	
Fuel oil consumption	On-site incineration	80 <sup>a</sup>	kWh tDM <sup>-1</sup>	
Biogas consumption	On-site incineration	740 <sup>a</sup>	kWh tDM <sup>-1</sup>	
Chemicals for sludge treatment	Copolymer of acrylamide, 0.48%)	0.22	kg kgDM <sup>-1</sup>	
	Quarry sand for filtration	0.028	kg kgDM <sup>-1</sup>	
Chemicals for on-site incineration	Calcium chloride	0.00065	kg kgDM <sup>-1</sup>	
	TMT 15/Na 3T, 15%	0.16	kg kgDM <sup>-1</sup>	
	Sodium persulfate	0.00064	kg kgDM <sup>-1</sup>	
	Sodium hydroxide	0.034	kg kgDM <sup>-1</sup>	
	NH <sub>3</sub> , 45%	5.07	kg kgDM <sup>-1</sup>	
	HCl	0.002	kg kgDM <sup>-1</sup>	
	Sulphuric acid	0.00120	kg kgDM <sup>-1</sup>	
	Sodium chloride	0.00059	kg kgDM <sup>-1</sup>	
	CO <sub>2</sub>	Anaerobic digestion	82.1	Nm <sup>3</sup> CO <sub>2</sub> tDM <sup>-1</sup>
	CH <sub>4</sub>	Anaerobic digestion	115.5	Nm <sup>3</sup> CH <sub>4</sub> tDM <sup>-1</sup>
Organic and inorganic	Incineration	Check in Larsen et al. (2010)	–	
Residue	Slag from incineration	515	kg tDM <sup>-1</sup>	

<sup>a</sup> Energy content.  
(Source: Larsen et al., 2010).

operates at a constant aeration flow rate, constant wastage flow rate and no chemicals are added in the biological treatment. Although the performance of this system is not optimal, it would accomplish with current legislation in terms of effluent nitrogen. However, phosphorous removal is minimal because of the limited available COD at the influent of the biological reactor, which results in competition between denitrification and phosphorous removal. Then, sixteen combinations of seven control algorithms [ $A = (A_2, \dots, A_{17})$ ] were implemented (See details in the [Supporting Information](#) and find comprehensive information about control of WWTPs in [Olsson et al., 2005](#)). The dissolved oxygen (DO) control at 2 gO<sub>2</sub> m<sup>-3</sup> aims at optimizing aeration energy consumption by maintaining the DO concentration in Ox1, Ox2 & Ox3 constant. The ammonium (NH<sub>4</sub><sup>+</sup>) cascade control sets NH<sub>4</sub><sup>+</sup> concentration in the last aerobic reactor (Ox3) to 1 gN m<sup>-3</sup> by manipulating the DO control set-point. Thus, during high influent NH<sub>4</sub><sup>+</sup> loads the DO set-point established by the controller is higher and during low influent loads it is lower. The third controller is based on oxygen uptake rate (OUR) measures. The controller switches the aeration in Ox1, 2 & 3 Off when the system reaches endogenous conditions (an indication that there is no nitrogen or organic matter to oxidize), which are assumed achieved at a measured OUR of 650 gO<sub>2</sub> m<sup>-3</sup> d<sup>-1</sup> in Ox1. When the aeration is On, the DO control at 2 gO<sub>2</sub> m<sup>-3</sup> is activated. The mixed liquor suspended solids (MLSS) controller maintains the desired biomass concentration in the reactors (during winter 3500 gTSS m<sup>-3</sup> and during summer 4000 gTSS m<sup>-3</sup>) by adjusting the wastage flow rate (Q<sub>w</sub>). The phosphate (PO<sub>4</sub><sup>3-</sup>) control aims at maintaining the PO<sub>4</sub><sup>3-</sup> concentration in Ox3 at 1 gPO<sub>4</sub>-P m<sup>-3</sup> by adding ferric chloride. Finally, the last set of controllers aims at maintaining the nitrate (NO<sub>3</sub><sup>-</sup>) in Ax2 at 1 gN m<sup>-3</sup> by manipulating either the internal recirculation (Q<sub>int</sub>) or the flow of sodium acetate addition (Q<sub>carb</sub>).

Fig. 2 illustrates the behaviour of some of the proposed controllers. Fig. 2a demonstrates that enhanced P precipitation with chemicals allows significantly reducing the PO<sub>4</sub><sup>3-</sup> concentration down to 1 g P m<sup>-3</sup>. Fig. 2b shows that NO<sub>3</sub><sup>-</sup> in Ax2 can be maintained at around 1 gN m<sup>-3</sup> with proper manipulation of the internal recycle flow. Fig. 2c illustrates the benefits of including DO control by optimizing the use of aeration (which in the default strategy is poor at daytime and excessive at night time with values up to

5 gO<sub>2</sub> m<sup>-3</sup>). Finally, Fig. 2d shows that implementing NH<sub>4</sub><sup>+</sup> control has a smoothing effect on the peaks as higher DO concentrations in the reactor are imposed during NH<sub>4</sub><sup>+</sup> peaks.

### 3. LCA goal and scope

The goal of the LCA part is to compare the environmental impacts of different control options modelled with the Neptune Simulation Benchmark (NSB). The scope of the LCA involves the construction and operation of the WWTP presented in Fig. 1 including the disposal of the residues. The infrastructure (material stage, construction etc.) is considered in the analysis although we assumed that no extra equipment was needed to implement the different controllers. The functional unit is the treatment of one cubic metre wastewater. The transport of the final residue to landfill was considered as well as the transport from the production site of the materials/chemicals to the WWTP. The variables for the LCA evaluation include direct gaseous (e.g. CO<sub>2</sub>, CH<sub>4</sub>, N<sub>2</sub>O), and aqueous (e.g. TN, TP) and solid (slag) emissions from the biological treatment and the sludge handling. The emissions and resources required for e.g. generating electricity to run the entire WWTP, for producing the chemicals applied to the biological treatment (sodium acetate and ferric chloride) and to sludge treatment (see Table 2) are also incorporated into the evaluation. The CO<sub>2</sub> emissions from the biological treatment step include only the non-biogenic carbon dioxide emissions from sodium acetate dosing, and not the biogenic carbon dioxide emissions (e.g. the CO<sub>2</sub> emissions generated from the oxidation of influent wastewater organics) as recommended by IPCC (2006).

### 4. Life cycle inventory

The inventory data is shown in Table 3 and was obtained after running simulations with the benchmark model NSB and applying the controllers described above. Emissions and consumptions related to the sludge handling can be estimated by using the sludge production- and emission factors listed in Table 2. It was assumed that sodium acetate was produced by mixing acetic acid (based on butane) with sodium hydroxide. The production of ferric chloride was based on the reaction of iron and hydrochloric acid. Data

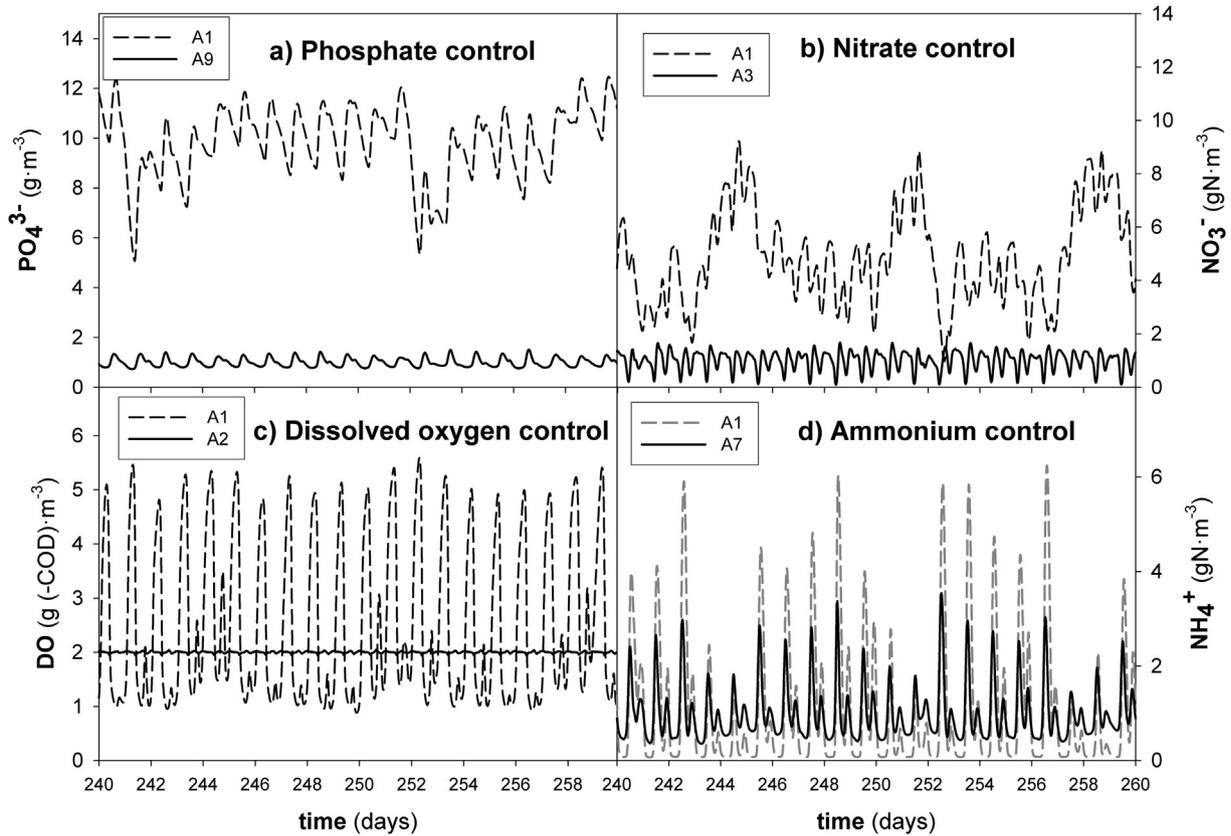


Fig. 2. Behaviour of the different controllers studied: Phosphate controller with chemical addition (a), Nitrate controller by means of internal recycle (b), DO controller (c), Ammonium controller (d).

regarding the production of the other chemicals were taken from the Ecoinvent database (Ecoinvent, 2007).

The inventory shows that by implementing a DO control, electricity consumption in the water line is reduced slightly (5%) from 0.22 to ~0.21 kWh m<sup>-3</sup> (values expressed per 1 m<sup>3</sup> of treated wastewater), while maintaining similar nutrient loads in the effluent (TN<sub>e</sub> and TP<sub>e</sub>). With the NH<sub>4</sub><sup>+</sup> controller, both electricity

consumption and TN<sub>e</sub> are decreased compared to the reference situation (by 18% and 13% respectively). The OUR controller allows reducing energy consumption by 9%. The MLSS controller does not always show a clear benefit since the sludge retention time is not a limiting factor for nutrient removal in the studied system. The PO<sub>4</sub><sup>3-</sup> control achieves a large reduction in TP<sub>e</sub> from 9.47 to 1.20 gP m<sup>-3</sup>. These controllers not only increase denitrification rates but also

Table 3  
Results of some of the evaluation criteria for the 17 evaluated control strategies.

Strat	TN <sub>e</sub>	TP <sub>e</sub>	Sodium acetate	Ferric chloride	Sludge production	Electricity water line	Electricity water line	Total CO <sub>2</sub> emissions
Unit	gN m <sup>-3</sup>	gP m <sup>-3</sup>	kg m <sup>-3</sup>	kg m <sup>-3</sup>	kgDM m <sup>-3</sup>	kWh m <sup>-3</sup>	kgCO <sub>2</sub> e m <sup>-3</sup>	kgCO <sub>2</sub> e m <sup>-3</sup>
A <sub>1</sub> Reference	13.19	9.47	0.00	0.00	0.39	0.22	0.14	0.15
A <sub>2</sub> DO	12.99	9.48	0.00	0.00	0.39	0.21	0.13	0.15
A <sub>3</sub> DO + Q <sub>intr</sub>	12.70	9.09	0.00	0.00	0.39	0.20	0.12	0.14
A <sub>4</sub> Q <sub>intr</sub> + NH <sub>4</sub>	11.48	8.15	0.00	0.00	0.40	0.18	0.11	0.14
A <sub>5</sub> Q <sub>intr</sub> + NH <sub>4</sub> + M <sub>LSS</sub>	11.26	8.02	0.00	0.00	0.40	0.18	0.11	0.14
A <sub>6</sub> DO + Q <sub>carb</sub>	9.25	5.73	0.08	0.00	0.42	0.24	0.15	0.32
A <sub>7</sub> Q <sub>carb</sub> + NH <sub>4</sub>	8.66	5.31	0.07	0.00	0.43	0.23	0.14	0.29
A <sub>8</sub> Q <sub>carb</sub> + NH <sub>4</sub> + M <sub>LSS</sub>	9.44	5.84	0.05	0.00	0.43	0.22	0.14	0.26
A <sub>9</sub> DO + PO <sub>4</sub>	12.88	1.20	0.00	0.049	0.40	0.21	0.13	0.30
A <sub>10</sub> PO <sub>4</sub> + NH <sub>4</sub>	11.94	1.19	0.00	0.045	0.41	0.20	0.12	0.28
A <sub>11</sub> PO <sub>4</sub> + NH <sub>4</sub> + M <sub>LSS</sub>	11.87	1.19	0.00	0.044	0.41	0.20	0.12	0.28
A <sub>12</sub> Q <sub>intr</sub> + OUR	12.42	8.88	0.00	0.00	0.39	0.20	0.12	0.14
A <sub>13</sub> Q <sub>intr</sub> + OUR + M <sub>LSS</sub>	12.61	8.86	0.00	0.00	0.39	0.20	0.12	0.15
A <sub>14</sub> Q <sub>carb</sub> + OUR	9.27	5.77	0.10	0.00	0.42	0.24	0.15	0.37
A <sub>15</sub> Q <sub>carb</sub> + OUR + M <sub>LSS</sub>	10.15	5.58	0.09	0.00	0.43	0.24	0.15	0.34
A <sub>16</sub> PO <sub>4</sub> + OUR	12.37	1.19	0.00	0.047	0.40	0.21	0.13	0.29
A <sub>17</sub> PO <sub>4</sub> + OUR + M <sub>LSS</sub>	12.74	1.18	0.00	0.046	0.40	0.21	0.13	0.29

Strat: strategy; TN<sub>e</sub>: effluent total nitrogen; TP<sub>e</sub>: effluent total phosphorus; DO: dissolved oxygen control; Q<sub>intr</sub>: internal recycle control, NH<sub>4</sub>: cascade ammonium control; M<sub>LSS</sub>: mixed liquor suspended solids control; Q<sub>carb</sub>: carbon addition control; PO<sub>4</sub>: phosphate addition control; OUR: oxygen uptake rate control; DM: Dried Matter. Values are expressed per m<sup>3</sup> of treated wastewater.

improve phosphorus removal. While  $Q_{\text{int}}$  coincides with reductions of  $\text{TN}_e$  and  $\text{TP}_e$  of  $\sim 15\%$ ,  $Q_{\text{carb}}$  achieves reductions down to  $\sim 50\%$  for  $\text{TN}_e$  and down to  $40\%$  for  $\text{TP}_e$ . The addition of sodium acetate ( $Q_{\text{carb}}$ ) increases sludge production up to  $0.43 \text{ kgDM m}^{-3}$ , increases electricity consumption in the water line and provokes the largest  $\text{CO}_2$  emissions ( $0.37 \text{ kgCO}_2\text{e m}^{-3}$  in strategy  $A_{14}$ ). From the inventory data we can observe that high nutrient removal efficiencies increase the overall  $\text{CO}_2$  emissions as a trade-off.

## 5. Life cycle impact calculation

The impacts were calculated by using a version of the LCA modelling tool GaBi 4 (PE, 2008) with the integrated LCA database Ecolnvent 2.0. The EDIP 97 methodology (Hauschild and Wenzel, 1998) was used considering the following impact categories: acidification (AC), soil ecotoxicity (SE), water ecotoxicity (WE), global warming (GW), human toxicity via air (HTA), human toxicity via soil (HTS) human toxicity via water (HTW), nutrient enrichment (NE), stratospheric ozone depletion (OD) and photochemical ozone formation (PO).

The impact potentials for each variable were calculated based on the principles of life cycle impact assessment (LCIA), i.e. classification, characterisation, normalisation and weighting, as described in the LCA standard (ISO 14040, 2006). In order to evaluate the impacts for the site-dependent conditions for the NE category (further information in the following section), the characterization, normalization and weighting steps were calculated for three scenarios: N&P-deficient, P-deficient and N-deficient ecosystems. Nutrients that were not limiting for each scenario were not taken into account in the characterization of NE. For instance, in the case of freshwaters, characterization factors for ammonium and nitrates were 0 as they are P-limited ecosystems. For global warming calculations, the marginal technology for energy production selected was based on natural gas. All normalization references used in this study are based on data for Europe (EU-15) except for global warming and ozone depletion which are based on global data (Stranddorf et al., 2005) (see *Supp Information*). For the first part of this study it has been chosen to give the same weight to all impact potentials, i.e. 1. Since the EDIP 97 methodology is used, the weighted person-equivalents are called targeted, i.e. person-equivalents-targeted (PET) and the weighted impact potentials are therefore expressed in the unit milli-person-equivalents-targeted regarding one year (mPET year). Further details on the quantifications can be found in the *Supplementary Material*. Inventory data (Table 3) combined with the impact potentials (Table 3 in *Supplementary Material*) were then used to calculate the induced and avoided impacts for the evaluated control strategies compared to the reference situation  $A_1$  (see Fig. 3).

## 6. Life Cycle Assessment interpretation

The selection of the more appropriate nutrient control strategy highly depends on the site-specific conditions. Looking at the NE impact category it is important to discriminate the exposed ecosystems (where WWTPs discharge) according to the limiting nutrient (Hauschild et al., 2008), i.e. the one which limits the growth of primary producers. Phosphorus is the limiting factor for plant growth in many freshwater ecosystems, whereas nitrogen is more commonly the key limiting nutrient of marine waters. Other ecosystems such as coastal zones can be limited by either nitrogen or phosphorous or both (Gallego et al., 2010). Therefore, decisions about wastewater nutrient removal might change depending on the type of receiving water body. Therefore, the LCI analysis is repeated for three different scenarios: i) N&P-deficient, ii) P-deficient and iii) N-deficient systems.

### 6.1. N&P-deficient scenario (Fig. 3a)

In N&P-deficient systems any control action leads to reduced environmental impact since the avoided impacts of all strategies are larger than the induced impacts. The strategies including chemical addition (from  $A_6$  to  $A_{11}$  and from  $A_{14}$  to  $A_{17}$ ) present better results compared to the ones without chemical addition. This is mainly due to the significant reductions in the effluent N and P loads (see Table 3). The strategies showing best environmental performance comprise the addition of ferric chloride ( $A_9$ ,  $A_{10}$ ,  $A_{11}$ ,  $A_{16}$ ,  $A_{17}$ ), reaching the highest difference between avoided and induced impacts (around  $2 \text{ mPET year m}^{-3}$ ). Implementing control for reducing energy consumption (strategies  $A_2$ ,  $A_3$ ,  $A_4$ ,  $A_5$ ,  $A_{13}$  and  $A_{14}$ ) is beneficial but does not lead to optimal environmental performance. The highest avoided impact is NE and the highest induced impact is on HT (on soil and water) categories. HT increase is caused by i) the increased sludge production which increments the sludge treatment impacts and ii) the production of sodium acetate and ferric chloride.

### 6.2. P-deficient scenario (Fig. 3b)

Avoided impacts are an order of magnitude larger compared to the N&P-deficient scenario as a result of the normalization step. For N&P-deficient systems  $\text{nIP}_{\text{ne},1} = 269$  and  $\text{nIP}_{\text{ne},2} = 37.2$ , whereas for P-deficient systems  $\text{nIP}_{\text{ne},1} = 2464$  and  $\text{nIP}_{\text{ne},2} = 0$  (see *Supplementary Material* for the specific calculations), i.e.  $\text{nIP}_{\text{ne},1}$  for P-deficient systems is eight times larger. Again, any control actions promoting both energy optimization and nutrient removal result in better environmental performance. The best performance is achieved by the strategies adding ferric chloride to increase phosphorus removal at the WWTP with absolute differences between induced and avoided impacts of  $\sim 20 \text{ mPET year m}^{-3}$ .

### 6.3. N-deficient scenario (Fig. 3c)

This last scenario does not show large environmental benefits from the implementation of the proposed controllers. The best strategy is  $A_5$ , with an absolute difference of  $0.08 \text{ mPET year m}^{-3}$ . This strategy includes DO, internal recycle,  $\text{NH}_4^+$  and MLSS controllers, which reduce energy consumption and total nitrogen discharges (compared to  $A_1$ ). Implementing an OUR controller ( $A_{12}$  and  $A_{13}$ ) is not as effective as the  $\text{NH}_4^+$  controller for this case. The strategies with sodium acetate addition do not always result in larger avoided impacts compared to the induced ones. Therefore, it is shown that there is not always environmental benefit in adding sodium acetate to decrease total nitrogen loads in the effluent. In the same way, the addition of ferric chloride for P removal has a negative impact on the environment in N-deficient scenarios.

Overall, NE is a dominating factor which has a strong influence on the selection of best operating strategies. This can be explained by the relatively high share of the NE normalization reference, which is related to WWTP emissions, whereas other normalization references (e.g. GW) are dominated by other societal activities like transport. As there is high uncertainty in the  $\text{N}_2\text{O}$  emission factors LCIA calculations were also conducted using the much higher factor  $0.03 \text{ kg N}_2\text{O kgTN removed}^{-1}$  (Foley et al., 2008). The obtained results (see *supporting Information*) showed that although the total  $\text{CO}_2$  equivalent emissions were almost doubled, no change in the final LCA outcomes was observed.

## 7. The importance of LCA weighting in decision-making

In order to reflect the relative importance of different impact categories on the results, weights can be assigned after the

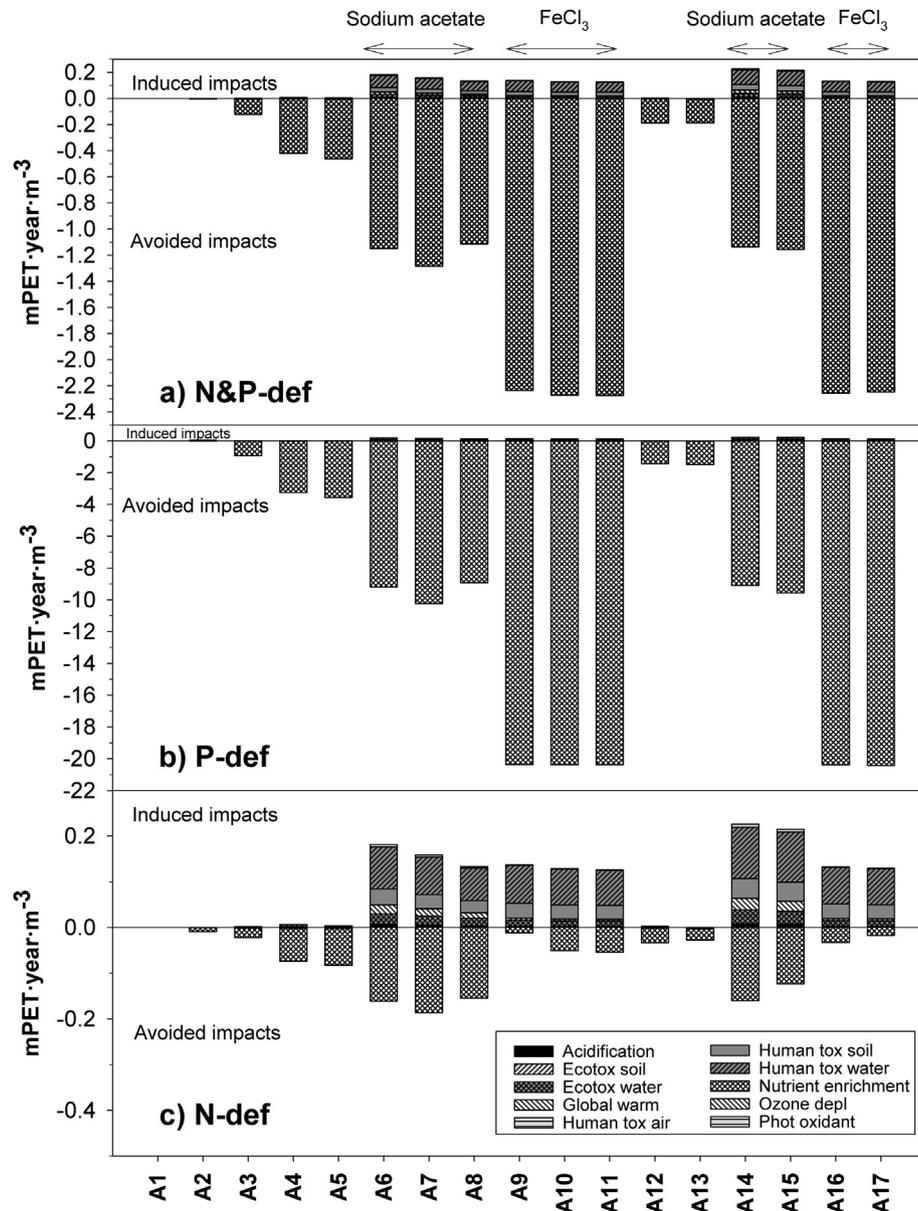


Fig. 3. Impact for each of the categories considered for the strategies A<sub>1</sub> to A<sub>17</sub>. a) N&P-deficient scenario, b) P-deficient scenario, c) N-deficient scenario.

normalization step. In general terms the criterion for weighting addresses the assumption that potential impacts in one impact category may be considered as more severe than potential impacts in other impact categories. Using a general Weighting Factor (WF) of 1 for all impact categories means that the severity of all categories is considered equal. However, weighting factors may also be determined by a method called 'distance to political reduction targets', i.e. a ratio between the potential impact in the reference year (i.e. the year of normalisation) and the potential impact in the target year (i.e. the year at which the reduced impact should be achieved) as described for the EDIP method (Hauschild and Wenzel, 1998).

In this study we test whether it is important or not to care about the weighting by performing a global sensitivity analysis (GSA, Neumann, 2012) on the weights of the impact categories. For the GSA, Monte Carlo calculations (200 iterations) were conducted by specifying ranges for the weighting factors, sampling from these ranges and propagating the sampled values through the LCIA model. The weighting factors were characterized using uniform probability

density functions, with the minimum values set at 1 and the maximum values set at the values provided by Stranddorf et al. (2005) using the 'distance to political reduction targets' approach. For global impacts (GW and OD) the worldwide weighting factors were used, whereas for regional and local impacts the EU-15 weighting factors were applied. The specific GSA method used is based on calculating standard regression coefficients (SRCs) between the GSA inputs (the weighting factors) and the outputs, i.e. the Net Environmental Impact (NEI). The NEI is the difference between the induced and the avoided impacts. Negative values indicate that the impact is reduced compared to A<sub>1</sub> while positive values correspond to increased impacts. The higher the absolute values of the SRC, the stronger the influence of the corresponding weight in determining the NEI.

Fig. 4 shows the absolute SRC values (from 0 to 1) for the three scenarios and for each strategy (from A<sub>2</sub> to A<sub>17</sub>). It can be seen clearly that NE is the impact category that is most sensitive to the weighting used (SRC values close to 1, in the outer circle), especially for N&P and P-deficient systems. However, for N-deficient systems and in

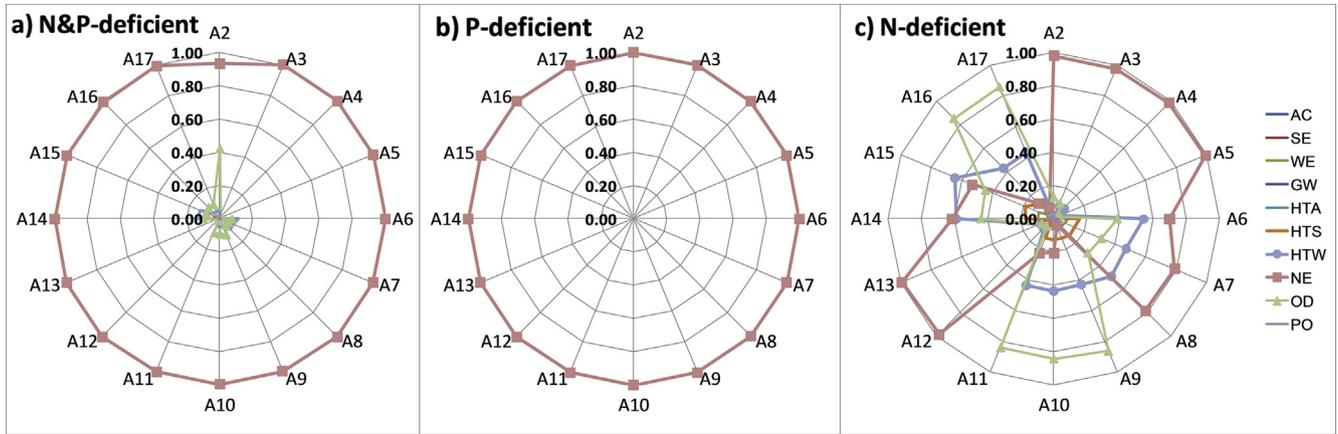


Fig. 4. Standard Regression Coefficients from the GSA on the weights of the impact categories. Ranges for the weights are: AC = [1–1.27], SE = [1–1], WE = [1–1.18], GW = [1–1.05], HTA = [1–1.4], HTS = [1–1.23], HTW = [1–1.3], NE = [1–1.22], OD = [1–63], PO = [1–1.33]. Lines which are not visible obtained SRCs close to 0.

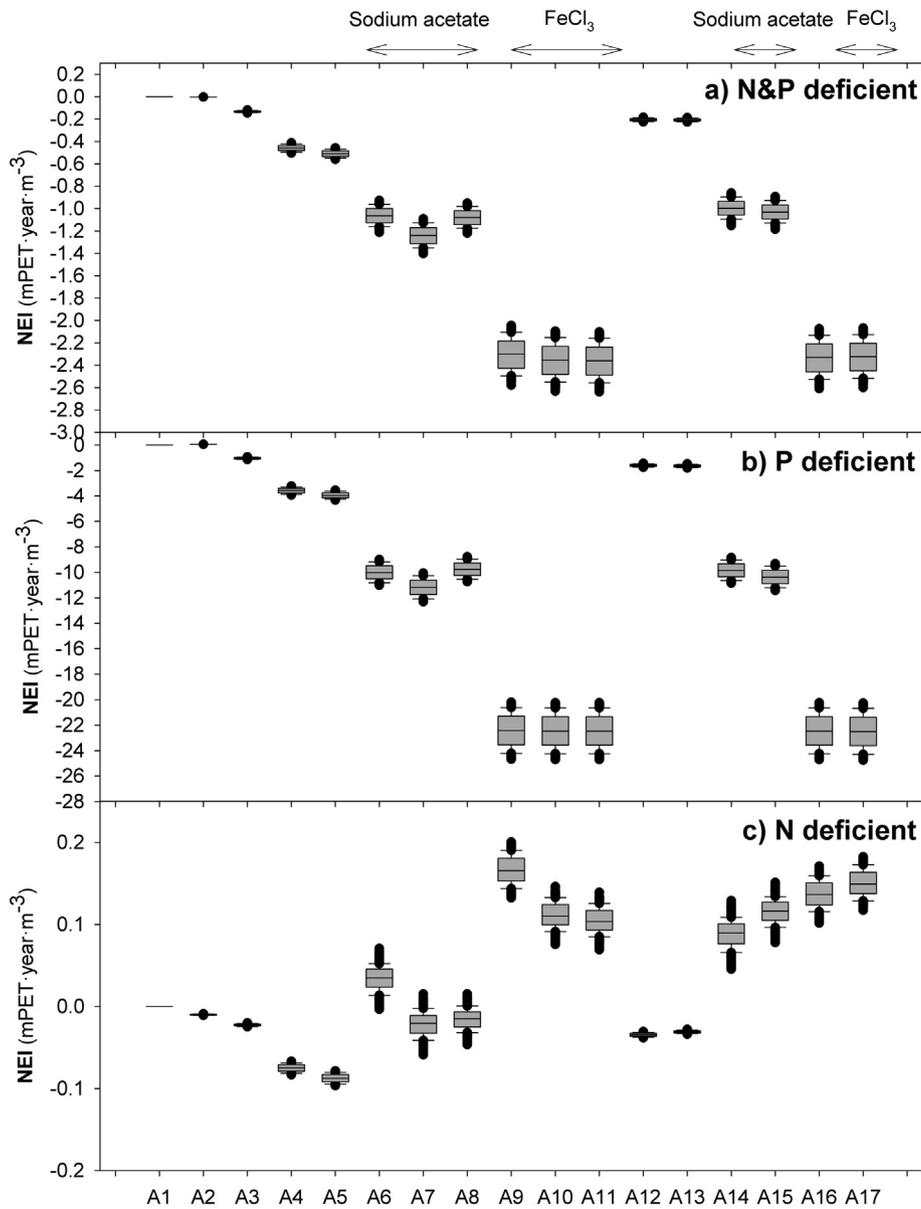


Fig. 5. Sensitivity of Net Environmental Impact to weighting factors for the strategies A<sub>1</sub> to A<sub>17</sub>. a) N&P-deficient scenario, b) P-deficient scenario, c) N-deficient scenario.

particular the strategies with ferric chloride addition (A<sub>9</sub>, A<sub>10</sub>, A<sub>11</sub>, A<sub>16</sub> and A<sub>17</sub>) changes in the NE weighting do not imply changes in the NEI. For these strategies, HTW and HTS impacts become the most sensitive to the weighting factors. As the evaluated LCIA model is linear all  $r^2$  values obtained from the regression were 1.

Fig. 5 shows the NEI for the three different scenarios, after propagating variability in the weighting factors through the LCIA model. It can be seen that the choice of weighting does not influence the selection of the best strategy for N&P and P-deficient scenarios since the interquartile ranges of the box plots do not overlap with competing groups of strategies. It is confirmed that best strategies involve ferric chloride addition. These interquartile ranges are larger for the strategies leading to largest reductions in NE, which is the most sensitive impact category. For N-deficient scenarios the selection of the weights has an effect on the ranking of some strategies (i.e. A<sub>6</sub>, A<sub>7</sub>, A<sub>8</sub>). In these cases, some combinations of weights lead to negative NEI values (being better strategies compared to A<sub>1</sub>), but for other combinations positive NEI values are obtained. It is clear though, that A<sub>4</sub> and A<sub>5</sub> are the best strategies for N-deficient scenarios.

## 8. Conclusions

Decision-making in controlling wastewater nutrient removal systems can be assessed using a combination of mechanistic process models together with LCIA models. The use of site-specific conditions for the NE impact category is essential to define best environmental performance strategies. In the evaluated case-study, strategies that promote increased nutrient removal and/or energy savings in WWTPs present an environmental benefit for N&P and P-deficient systems. This is not the case in N-deficient systems where the use of chemicals (even for improving N removal efficiencies) would not always be beneficial for the environment. A sensitivity analysis applied to the weights of the impact categories showed that the choice of weighting factors would only influence decision making for N-deficient systems.

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## Appendix A. Supplementary data

Supplementary data related to this article can be found at <http://dx.doi.org/10.1016/j.jenvman.2013.06.002>.

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